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DISCUSSION OF SETTLEMENT CORRECTION AT LA GUARDIA FIELD (Published in November, 1950)

By D. P. Krynine, and John M. Kyle

AIR TRANSPORT DIVISION

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Structural	42, 49, 51, 53, 54, 59, 61, 66, 89 (Discussion: D-5, D-3, D-8, D-16, D-23, D-13, D-21, D-24, D-25-K, D-32, D-17, D-33, D-34, D-39, D-42)
Surveying and Mapping	50, 52, 55, 60, 63, 65, 68
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## DISCUSSION

D. P. KRYNINE, M. ASCE.—Since the construction of La Guardia Airport (New York, N. Y.), American engineers have been extremely interested in developments at that field, especially concerning the large settlements in the area. This paper satisfies to a large degree the scientific curiosity of engineering circles, though some details require still further elucidation.

After giving some historical and geological information on the La Guardia Field area, the author presents, among other things, the following two basic items: (1) experimentation on the three large sand drain areas; and (2) construction of the dike surrounding that area of the airport in which the maximum settlement had occurred. The writer's comments refer to these two items only.

The general principle of the sand drain method consists of acceleration of the rate of settlement that may be expected due to the presence of a soft substratum. Since there is no possibility of eliminating such settlement, at least at the present (1951) state of the foundation technique, the sooner this settlement takes place, the better. With this criterion in mind the writer examined the comparative merits of different drain spacings used in the experimental areas (Segments I, II, and III). It appears from the data presented in Fig. 2 and Table 2 that during the whole time of experimentation there was a continuous lag in the behavior of Segment I with the largest drain spacing (14 ft) as compared with Segment II (11 ft drain spacing). Not only was the settlement of Segment I smaller than that of Segment II during the whole time of experimentation, but the final results differed by about 15% (6.43 ft settlement for Segment I and 7.55 ft settlement for Segment II). On the other hand, although in the first year of experimentation there was some difference in the behavior of Segments II and III (11-ft and 8-ft spacings of sand drains, respectively), close to the end of the first year this difference became negligible. The final settlement observed in Segment II was even a little larger than that in Segment III.

The writer's conclusion from the experimental results is that 14-ft spacing of drains (Segment I) under the conditions at La Guardia Field proved to be somewhat excessive, whereas the 8-ft spacing (Segment III) showed a wasteful crowding of drains. It is not clear from the paper what advantage was taken of the full-size experiment described, that is what was the actual drain spacing in the final design and construction of the dike. Another thing that interests the writer is whether some attempt was made to correlate the spacing of the drains and the results shown in Fig. 2 and Table 2 with the permeability (particularly with the ratio of the vertical and horizontal permeabilities) of the soil material or materials at La Guardia Field. Such information, if available, would be of extreme value to engineers who may find themselves facing a settlement problem similar to that described in this paper.

Note.—This paper by John M. Kyle was published in November, 1950, as Proceedings-Separate No. 43. The numbering of footnotes and illustrations in this Separate is a continuation of the consecutive numbering used in the original paper.

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The writer hopes that in his final closure Mr. Kyle will give some additional information with reference to the questions offered in this discussion. Some numerical data on the dimensions of the drains (length, diameter) and the cross section of the dike would also be of interest.

JOHN M. KYLE<sup>8</sup>.—The discussion by Mr. Krynine has emphasized an important point, concerning the spacing of drains, that should clarified. By examination of Fig. 2 and Table 2 only, one could conclude that the 8-ft spacing was a wasteful crowding of drains.

Because of the limited space available at the airport, it was unfortunately necessary to have the segments of the test section adjacent to each other. Fig. 3 shows the layout of the test section and the settlement curve for each segment up to April, 1951. The drains in Segment I were spaced 14 ft on

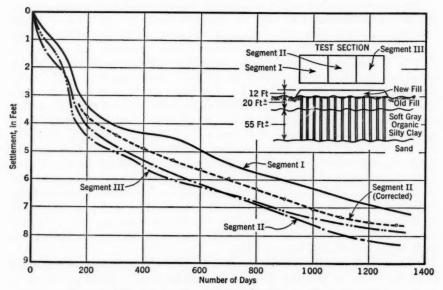


Fig. 3.—Settlement Observations on Sand Drains for Subsoil Stabilization

centers. Drain spacing in Segments II and III was 11 ft and 8 ft on centers, respectively. From the layout of the test section, it can be seen that Segment II, being surrounded on each side by one of the other segments, has a greater load acting on it than either Segment I or Segment III. By correcting for this additional load, a new settlement curve can be plotted for Segment II, and this curve is shown as a dashed line in Fig. 3. This corrected curve now takes a more proper place in relation to the other curves and would tend to prove that, under the same loading conditions, a quicker rate of settlement can be achieved by a smaller spacing of drains.

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The ratio of horizontal permeability  $(K_h)$  to vertical permeability  $(K_v)$  was constant throughout the test section area, although it differed slightly throughout the airport. Because of this, no correlation between rates of settlement of different spacings of drains and different  $K_h/K_v$  ratios could be attempted. However, based on test section results and the theory of radial drainage, it is believed that any increase in  $K_h/K_v$  ratios would result in a relative increase in the rate of settlement, and that this increase would be constant for the different spacings of sand drains.

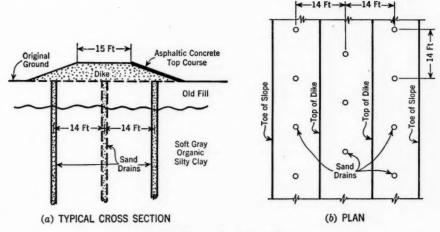
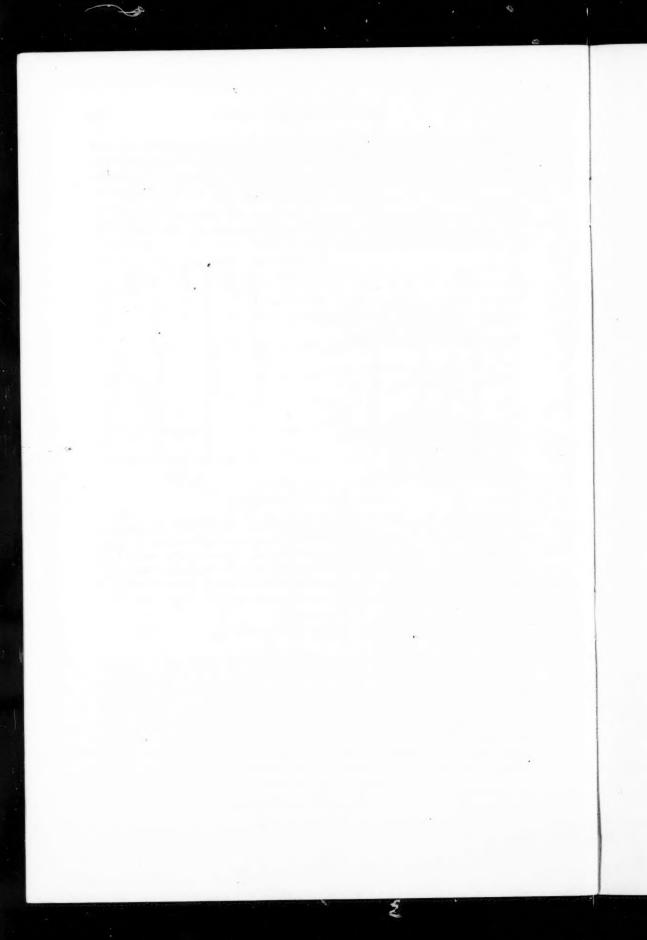


Fig. 4.-Dike at La Guardia Field

Fig. 4 shows a typical cross section of the dike. Sand drains were installed in three rows at 14-ft spacing in the area to be occupied by the dike. A larger number of rows or a smaller interval of spacing of sand drains would have become unduly expensive and was not deemed necessary because of the nature and purpose of the dike. The sand drains were 18 in. in diameter and were approximately 80 ft long.



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